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StructuresNet and *FireNet*: Benchmarking Databases and Machine Learning Algorithms in Structural and Fire Engineering Domains

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19	······································
20	Abstract
20	

Machine learning (ML) continues to rise as an effective and affordable method of tackling 21 engineering problems. Unlike other disciplines, the integration of ML into structural and fire 22 engineering domains remains deficient. This is due in part to the lack of benchmark databases to 23 compare the effectiveness of ML models. In order to bridge this knowledge gap, this paper 24 presents a benchmark examination of common supervised learning ML algorithms that can be 25 easily deployed into structural and fire engineering problems. The selected algorithms include; 26 Decision Trees (DT), Random Forest (RF), Extreme Gradient Boosted Trees (ExGBT), Light 27 Gradient Boosted Trees (LGBT), TensorFlow Deep Learning (TFDL), and Keras Deep Residual 28 Neural Network (KDP), and are used with their default values to establish a proper benchmark 29 against six databases. The compiled datasets have been thoroughly tested and span two domains, 30 structural engineering; 1) elemental response of concrete-filled steel tubular (CFST) circular 31 columns at ambient conditions, 2) shear response of cold-formed steel (CFS) channels with 32 slotted webs, 3) compressive strength of concrete, 4) fatigue life data, 5) shear strength of 33 reinforced concrete (RC) beams and FRP-strengthened RC beams; and fire engineering, 6) fire 34 behavior of RC concrete columns in terms of spalling occurrence and fire resistance. This study 35 also investigates a variety of commonly used performance metrics that are applicable to 36 regression and classification-based ML problems. We invite ML users to apply their models to 37 the presented databases to establish a benchmark by mean of external validation and then extend 38 their models to other problems and databases. Collectively, the presented work establishes the 39 first step towards a unified framework that can be used to accelerate the adoption of ML into 40 structural and fire engineering domains. 41

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43 Keywords: Machine learning; Artificial intelligence; Validation; Databases; Structures; Fire.

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44 Introduction

Engineers tend to design experiments to examine problems [1]. Naturally, such experiments are 45 limited in their number of test specimens and parameters, and are also articulated to fit the 46 available testing equipment and facilities. To ensure comparability of tests, testing standards 47 (i.e., ASTM, ISO etc.) were established. Such standards provide a unified reference for test 48 methods, equipment, and specifications for various testing scenarios and environments. 49 Regardless of the test in question, the notion of a testing standard is to ensure compliance to a 50 certain procedure that allows duplication of results among different stakeholders. Within the 51 structural and fire engineering domains, testing standards primarily exist for material property 52 (e.g. testing for compressive strength of concrete etc.), and examination of elemental behavior 53 (to some extent) [2-4]. 54

In lieu of experiments, engineers may also utilize advanced numerical tools such as finite element (FE) methods to model material and elemental response at ambient or fire conditions. Advancements in such numerical tools were made possible as a result of developing improved computing workstations with fast processing units that can be obtained conveniently. In a way, numerical methods provide users with affordable and "logical" means to predict structural and fire engineering phenomena – noting how FE is founded upon solving partial differential equations which are formulated through functional minimization techniques [5].

A common practice of utilizing FE methods is to first validate predictions from a developed FE 62 model against that obtained from a real test. Such validation is often displayed in terms of a chart 63 with two series; thereby comparing predictions from FE model and measurements from 64 experiments [6,7]. The open literature seems to agree that a "good validation" is that which has a 65 5-20% variation between FE predictions and test measurements [8–13]. However, establishing a 66 good agreement is often subjective as we continue to lack a standardized method to establish 67 such validation. Despite the lack of a standardized procedure not only to develop a FE model, or 68 carry out a simulation but also to validate such a simulation, the use of FE modeling is regarded 69 as a cornerstone within the civil engineering industry [14–18]. 70

Still, a few questions might arise that may question the suitability of FE simulation as a tool. For 71 example, what are the recommended element types to be used in a specific FE model aimed to 72 explore a given phenomenon? What material models are considered proper to model such a 73 phenomenon? What convergence criteria and solution technique should one use in a modeling a 74 specific problem? Moreover, what constitutes a good FE model? And how can we ensure that a 75 developed FE model can be safely applied beyond its intended use or range of applicability? In a 76 way, and at this day and age, the use of FE modeling seems to be treated as propriety 77 information, and often regarded as an art with a complementary scientific component [19]. 78

The above also brings in a few questions in light of the rise of Machine learning (ML) as a potential new method for tackling structural and fire engineering problems [20–23]. Simply put, ML can be thought of ML is a universal method that can be applied to virtually any engineering problem. In a way, ML can be thought of as a software that can be applied to explore the observations noted in our databases. Thus, the following questions may primarily be of

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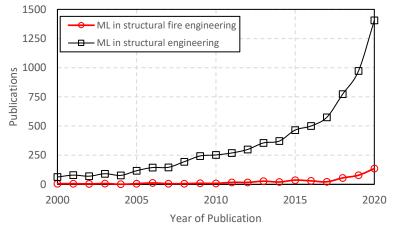
Naser M.Z., Kodur V.K.R., Thai H, Hawileh R, Abdalla J, Degtyarev V. (2021). "StructuresNet and FireNet: Benchmarking Databases and Machine Learning Algorithms in Structural and Fire Engineering Domains." *Journal of Building Engineering*. <u>https://doi.org/10.1016/j.jobe.2021.102977</u>.

importance, especially to researchers and practicing engineers interested in adopting ML. For 84 example: What algorithms can one use? Are all algorithms the same? Do algorithms need to be 85 developed from scratch? Or can existing algorithms (pre-developed) be used as is? Where to get 86 data to develop ML models? How to validate such models? As one can see, these questions 87 mirror those outlined above and others not mentioned herein for brevity (e.g., what coding 88 language to use?). The authors of this work believe that addressing the above questions early and 89 during the current rise of ML will not only be beneficial to this community but will help 90 facilitate the adoption of ML as a new method of choice. As such, the primary motivation behind 91 this work builds upon similar calls for developing validation and benchmarking procedures for 92 FE models [24–27] and aims to set the stage toward a unified ML procedure within structural 93 and fire engineering domains and by structural and fire engineers. 94

A closer look into the open literature shows that publications with a ML theme in structural 95 and/or fire engineering continue to steadily rise (see Fig. 1). Noting how parallel fields have 96 embraced ML indicates that ML will continue this positive trend. In addition to our examination 97 98 of global trends, a deep dive into the open literature highlights how ML has been successfully used in a variety of problems. For example, Behnood and Golafshani [28,29] developed a series 99 of ML models to examine properties of concrete derivatives (traditional concrete, concrete with 100 waste foundry sand, and high-performance concrete), and asphalt materials with notable success 101 and have led to creating new and simple models that can predict the properties of concrete and 102 asphalt materials. In addition, the works of Mangalathu et al. [30,31] showcased how ML models 103 can be used to predict the seismic and structural response of concrete shear walls and bridges, 104 which has also led to developing open-source classification models. Degtyarev [32,33] 105 successfully developed a database and an Artificial Neural Network (ANN) to examine the 106 response of shear strength of CFS channels with slotted webs with high accuracy exceeding 107 95%. The collaboration of Lopes and Bobadilha [34,35] has resulted in novel ML models that 108 were applied to evaluating the quality of timber materials. These models achieved notable 109 accuracy exceeding 75%, and were shown to be convertible into mobile phone applications. 110 111 Additional works that applied ML into this domain were also identified by other research groups [36–44], and carried out by the authors [45–52]. 112

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114 115

Fig. 1 ML-based publication trends during 2000-2020

This work aims to present a benchmarking study that applies commonly used supervised learning 116 algorithms (with default settings) to publicly available databases with a goal to establish a first 117 documentation and examination for validation of ML models. The selected algorithms include; 118 Decision Trees (DT), Random Forest (RF), Extreme Gradient Boosted Trees (ExGBT), Light 119 Gradient Boosted Trees (LGBT), TensorFlow Deep Learning (TFDL), and Keras Deep Residual 120 Neural Network (KDP). Furthermore, this paper showcases commonly used validation and 121 performance metrics that can be applied to regression and classification problems by examining 122 six large datasets covering concrete, and steel materials and structures, and conveniently named 123 StructuresNet and FireNet. 124

The intend of this paper is to outline a systematic procedure to maintain repeatability and 125 benchmarking of commonly used ML models within the structural and fire engineering domains. 126 We would like to emphasize that the goal of the shown analysis is not to finetune algorithms to 127 128 report upon the best performing algorithm to examine a particular problem, nor to guarantee tuning of algorithms to chase high metrics, but rather to apply the selected algorithms in their 129 default settings to allow interested readers from repeating this work to compare the performance 130 of their algorithms and then aim to develop improved models (both of which may encompass 131 similar or other types of algorithms to that used herein). By using algorithms in their default 132 settings, the attained performance of these algorithms is then "benchmarked" and documented. 133 Such benchmarking will allow future ML users from also benchmarking newly developed ML 134 models or ensembles and compare their performance against that of the most commonly used 135 models reported in our domains on the presented databases. We hope that this paper founds an 136 approach that can be further massaged by the collective works in our domains to establish a 137 uniform, and possibly standardized, mean to apply ML models to fully harness the positive 138 potential of this technology in the near future. The message of this work aligns with that 139 proposed by other researchers that focused on FE models [24–27]. 140

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141 **Description of Databases**

This section describes the examined six databases of varying *size* and *scope* within the structural 142 and fire engineering disciplines in details. These databases cover re-occurring problems of 143 simple and complex nature and fall under regression and classification problems. Information 144 with regard to the history of each database, and to statistical distribution are provided herein for 145 brevity. Please note that a more in-depth analysis on each database can be found in their 146 respective references [32,48,53–59]. Table 1 gives insights into the presented databases. One 147 should also note that there is very limited data on fire-exposed structural elements and structures; 148 which is reflected by the smaller size of *FireNet* as opposed to *StructuresNets*. All of the 149 presented databases are hosted online on Mendelev public repositories, as well as original papers 150 (and complete links to these databases are shown herein: Database 1 [59]. Database 2 [122]. 151

152 Database 3 [123]. Database 4 [63,64]. Database 5 [55,67]. Database 6 [54]).

Database	Domain	Application	Category	No. of data points	Basis	References	
Thai database		Design of CFST columns		3,103	170 tests	[58,59]	
Degtyarev & Degtyareva database		Shear strength of CFS channels with slotted webs		3,512	FE simulations	[32]	
Yeh database	Structural engineering (StructuresNet)	Compressive strength of high- performance concrete	Regression	1,030	tests	[57]	
Abdalla & Hawileh database		Fatigue life data		59	tests	[48]	
Abdalla et al. database		Shear strength of RC and FRP- strengthened beams		290	tests	[55,56]	
Naser & Kodur database	r & Fire ur engineering of reinforced		Regression and Classification (binary and multi-class)	306	140 tests and 169 FE simulations	[53,54]	

153 Table 1 Details on *StructuresNet* and *FireNet*¹

¹ A historical, rough and traditional rule of thumb is that a minimum set can be 10 cases per predictor per van Smeden et al. [124]. However, this rule of thumb has been associated with some limitations (as noted by Riley et al. [125]) and hence a revised 23 cases per predictor criteria is proposed. As of this moment, a series of investigation continue to be carried out to better answer the question of the minimum size of databases needed for a ML analysis. In all cases, our databases satisfy both, the traditional criterion, and newly proposed criterion.

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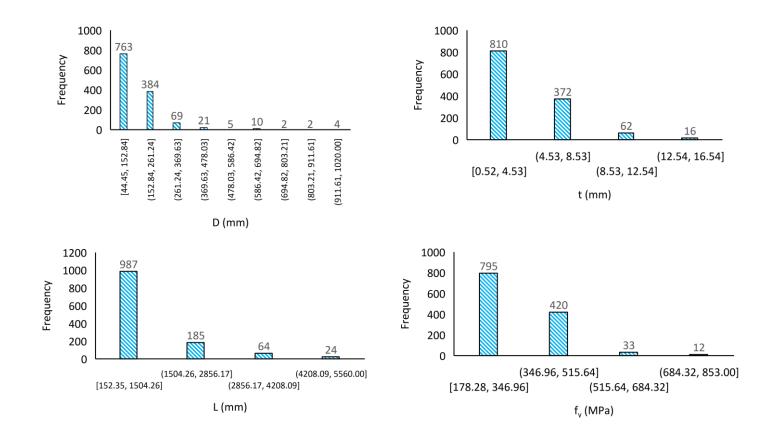
155 *Database on concrete-filled steel tubular (CFST) columns (StructuresNet 1: Thai database)*

A comprehensive database that covers four types of concrete-filled steel tubular (CFST) columns was developed by Thai et al. [58,59]. This database collected 3,103 notable tests on CFST columns collected from over 170 studies and falls under a regression database. The selected columns cover a range of configurations (short, slender, circular, square, and rectangular sections) that were tested under concentric and eccentric loading.

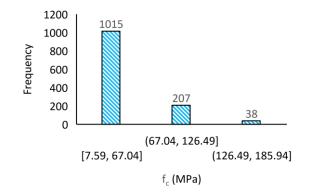
This database documents geometric features in terms of: 1) effective length, L_e , 2) tube 161 thickness, t, 3) tube diameter, D, and material properties in terms of 4) yield stress, f_{v} , 5) 162 compressive strength, f_c , of in-filled concrete of CFST columns. Other features were also 163 included such as modulus of concrete and steel, ultimate strength of steel section, load eccentrics 164 165 etc. For sake of this study, 1,245 circular CFST columns that were tested under concentric loading are examined herein. A graphical distribution of all features in this database is plotted in 166 Fig. 2 and Table 2 summarizes the main attributes of the collected database in terms of material 167 and geometric features. 168

Table 2 also shows that this database covers a practical range of CFST columns. For example, 169 170 the minimum and maximum diameters of circular columns range between 44.45 mm and 1020.00 mm. The thickness range of the same columns varies between 0.52-16.54 mm. The 171 range of yield strength of steel tubes and compressive strength of concrete filling is from 9.17 172 MPa to 193.30 MPa for concrete, and from 115.00 MPa to 853.00 MPa for steel. A sensitivity 173 analysis was carried out to identify a correlation between all features compiled in this database. 174 The outcome of this analysis shows that of all features, geometric features (D, and t) are of the 175 highest importance. One should note that this sensitivity analysis is independent of the used ML 176 model. 177

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Fig. 2 Frequency of identified features of selected CFST in the compiled database

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Section	Features	D (mm)	t (mm)	D/t	Le (mm)	$f_y(MPa)$	f'c (MPa)
	Min	44.45	0.52	7.42	152.35	178.28	9.17
	Max	1020.00	16.54	220.93	5560.00	853.00	193.30
Circular	Average	158.52	4.31	44.28	1060.53	336.35	50.21
(concentric loading)	Standard deviation	105.42	2.45	32.37	1005.28	90.89	31.57
	Median	127.3	4.00	33.33	662.00	325.00	41.00
	Skewness	3.71	1.58	2.86	1.98	2.18	2.06
Param	Parameter		f_c	f_y	Le	t	N
D		1.000					
f_c		-0.003	1.000				
f_y		0.072	0.030	1.000			
Le		0.201	-0.154	0.080	1.000		
t		0.478	-0.022	0.238	0.216	1.000	
Ν		0.911	0.126	0.145	0.109	0.549	1.000

179 Table 2 Key statistics from CFST database.

180 N: axial capacity (kN).

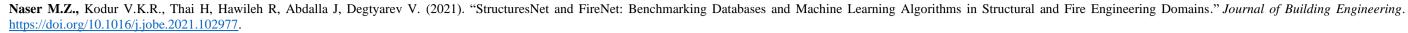
181 Database on shear strength of cold-formed steel (CFS) channels with slotted webs

182 (*StructuresNet 2: Degtyarev & Degtyareva database*)

The second database falls under a regression database and compiles observations taken from 3,512 FE simulations aimed to investigate the elastic shear buckling loads and the ultimate shear strengths of CFS channels with slotted webs as carried out by Degtyarev & Degtyareva [60–62], and recently published at [32]. In this database, the ultimate shear strengths of the CFS channels were determined from FE models that account for material and geometric nonlinearities, as well as initial geometric imperfections – thereby making this dataset rich with realistic information.

Overall, this database accounts for 15 features: 1) channel depth, D, 2) channel flange width, B, 189 3) channel flange stiffener length, B_1 , 4) channel thickness, t, 5) length of slots, L_{sl} , 6) height of 190 slots, W_{sl} , 7) spacing of slots in the longitudinal direction, S_{sl} , 8) spacing of slots in the transverse 191 direction, B_{sl} , 9) number of perforated regions, N, 10) number of slot rows, n, 11) yield stress of 192 steel, f_{y} , 12) type of boundary conditions: realistic and test setup (designated as 1 and 2, 193 respectively), 13) inside bend radius, r, 14) the aspect ratio, a/h, and 15) height of the 194 longitudinal stiffener, h_{st} , that can be used to predict the elastic shear buckling load, V_{cr} , and/or 195 the ultimate shear strength, V_n (see Fig. 3). In this benchmark study, the elastic shear buckling 196 load, V_{cr} will be solely used. The outcome of the sensitivity analysis is listed in Table 4 and 197 shows strong correlation between channel thickness and inside bend radius, and elastic shear 198 buckling load. It is worth noting that the inside bend radius was taken as 2t in all models, and 199 hence the strong correlation (noting that channel thickness has a strong correlation with V_{cr}). 200

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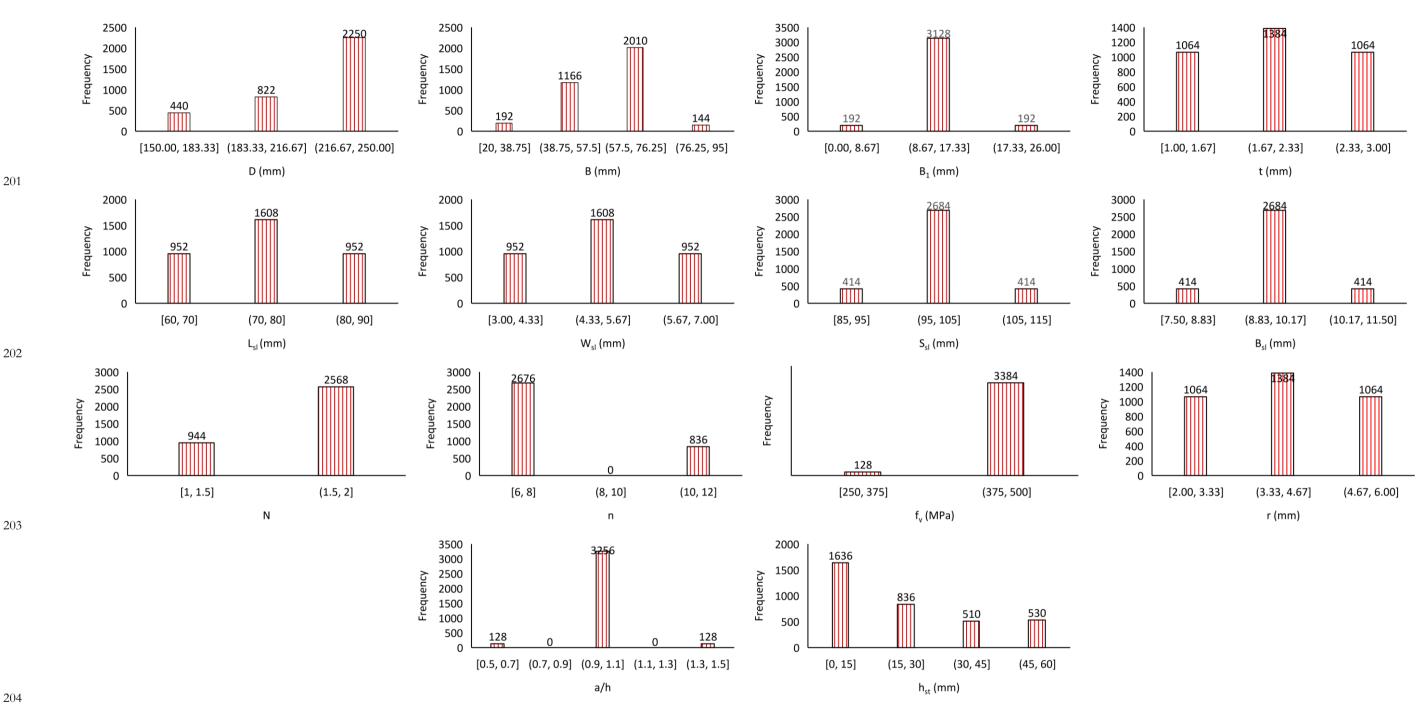


Fig. 3 Frequency of identified features of selected channels in the compiled database



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	D (mm)	B (mm)	B 1 (mm)	t (mm)	Lsl (mm)	W _{sl} (mm)	S _{sl} (mm)	Bsl (mm)	N	n	f_{y} (MPa)	BC	r (mm)	a/h	h _{st} (mm)	Vcr (N)
Minimum	150.0	20.0	0.0	1.0	60.0	3.0	85.0	7.5	1.0	6.0	250.0	0.0	2.0	0.5	0.0	401.9
Maximum	250.0	95.0	26.0	3.0	90.0	7.0	115.0	11.5	2.0	12.0	500.0	-	6.0	1.5	60.0	309322.4
Average	225.8	57.8	13.0	2.0	75.0	5.0	100.0	9.5	1.7	8.0	490.9	-	4.0	1.0	19.6	32107.1
Standard deviation	35.4	13.5	4.3	0.8	11.0	1.5	7.3	1.0	0.4	2.4	46.9	-	1.6	0.1	22.0	37675.0
Skewness	-1.1	0.2	0.0	0.0	0.0	0.0	0.0	0.0	-1.0	0.8	-4.9	-	0.0	0.0	0.7	2.3
							_	_		_				_		
Parameter	D (mm)	B (mm)	B_1 (mm)	t (mm)	L _{sl} (mm)	W _{sl} (mm)	S _{sl} (mm)	B _{sl} (mm)	Ν	п	f_y (MPa)	BC	r (mm)	a/h	h _{st} (mm)	$V_{cr}\left(N ight)$
D(mm)	1.000															
B (mm)	0.648	1.000														
$B_1(mm)$	0.000	0.000	1.000													1
t (mm)	0.000	0.000	0.000	1.000												
$L_{sl}(mm)$	0.000	0.000	0.000	0.000	1.000											
W_{sl} (mm)	0.000	0.000	0.000	0.000	0.000	1.000										
$S_{sl}(mm)$	0.000	0.000	0.000	0.000	0.000	0.000	1.000									
$B_{sl}(mm)$	0.000	0.000	0.000	0.000	0.000	0.000	0.000	1.000								
Ν	0.218	0.131	0.000	0.000	0.000	0.000	0.000	0.000	1.000							
п	0.370	0.248	0.000	0.000	0.000	0.000	0.000	0.000	0.012	1.000						
$f_y(MPa)$	-0.133	-0.103	0.000	0.000	0.000	0.000	0.000	0.000	-0.008	-0.079	1.000					
BC	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	1.000				
r (mm)	0.000	0.000	0.000	1	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	1.000			
a/h	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	1.000		
$h_{st}(mm)$	0.311	0.200	0.000	0.000	0.000	0.000	0.000	0.000	0.539	0.131	-0.003	0.000	0.000	0.000	1.000	
$V_{cr}(N)$	-0.058	-0.0298	0.004	0.726	-0.306	-0.130	0.061	0.078	0.020	-0.118	-0.031	0.0981	0.726	-0.1474	0.012	1.000

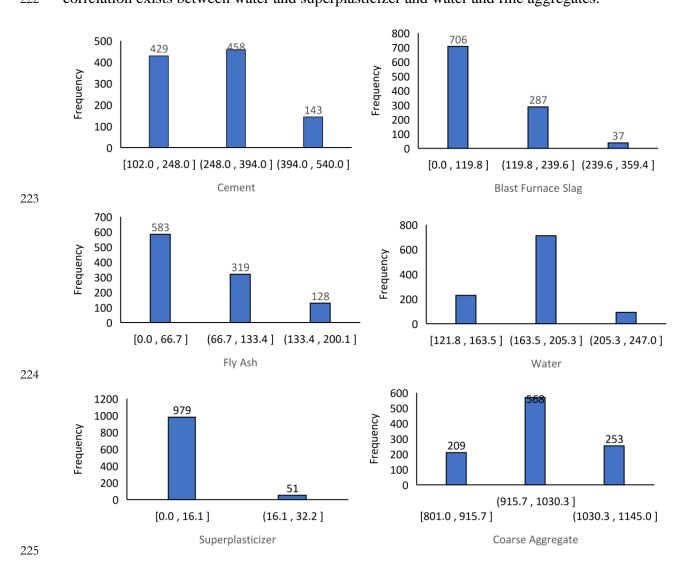
Table 4 Statistics from collected database.

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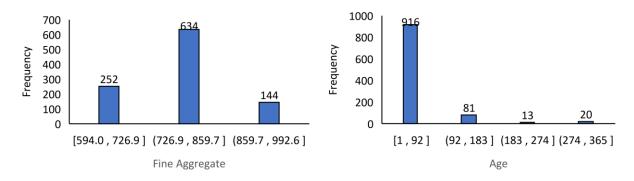
212 Database on compressive strength of high-performance concrete (StructuresNet 3: Yeh 213 database)

The third database falls under a regression database and compiles 1,030 data points taken from 214 tests that determined compressive strength (f'_c) of high-performance concrete (HPC) as a 215 function of: 1) Cement, C, 2) Blast Furnace Slag, B, 3) Fly Ash, F, 4) Water, W, 5) 216 Superplasticizer, S, 6) Coarse Aggregate, CA, 7) Fine Aggregate, FA, and 8) Age, A. This 217 database was published by Yeh [57] and has been extensively used in ML studies. Figure 4 and 218 Table 5 show the distribution of all features comprising this database. The outcome of the 219 correlation matrix shows the highest positive correlation to be between cement and compressive 220 strength, followed by age and compressive strength, and flay ash and superplasticizer. A negative 221 correlation exists between water and superplasticizer and water and fine aggregates. 222



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226

Fig. 4 Frequency of identified features of concrete mix designs in the compiled database

228

227

Section	Features	С	В	F	W	S	CA	FA	Α	f'^{c}
	Min	102.0	0.0	0.0	121.8	0.0	801.0	594.0	1.0	2.3
	Max	540.0	359.4	200.1	247.0	32.2	1145.0	992.6	365.0	82.6
Compressive	Average	281.2	73.9	54.2	181.6	6.2	972.9	773.6	45.7	35.8
strength of HPC	Standard deviation	104.5	86.3	64.0	21.4	6.0	77.8	80.2	63.2	16.7
	Median	0.5	0.8	0.5	0.1	0.9	0.0	-0.3	3.3	0.4
	Skewness	102.0	0.0	0.0	121.8	0.0	801.0	594.0	1.0	2.3
Param	eter	С	В	F	W	S	CA	FA	Α	f'_c
С		1.000								
В	В		1.000							
F		-0.397	-0.324	1.000						
W		-0.082	0.107	-0.257	1.000					
S		0.093	0.043	0.377	-0.657	1.000				
CA		-0.109	-0.284	-0.010	-0.182	-0.266	1.000			
FA		-0.223	-0.282	0.079	-0.451	0.223	-0.179	1.000		
A		0.082	-0.044	-0.154	0.278	-0.193	-0.003	-0.156	1.000	
f_c		0.498	0.135	-0.106	-0.290	0.366	-0.165	-0.167	0.329	1.000

Table 5 Statistics on collected database

230

231 Database on low cycle fatigue (StructuresNet 4: Hawileh & Abdalla database)

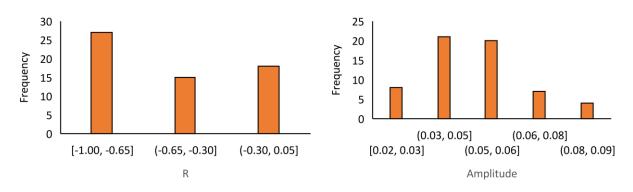
The fourth database falls under a regression database and compiles real observation of around 60 data points taken from strain-controlled low-cycle fatigue tests that were carried out on steel reinforcing bars under cyclic load with a frequency of 0.05 Hz. The tests determined the lowcycle fatigue life by measuring the number of reversals $(2N_f)$ to fatigue failure of steel

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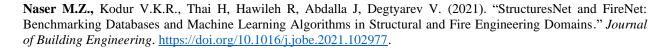
Naser M.Z., Kodur V.K.R., Thai H, Hawileh R, Abdalla J, Degtyarev V. (2021). "StructuresNet and FireNet: Benchmarking Databases and Machine Learning Algorithms in Structural and Fire Engineering Domains." *Journal of Building Engineering*. <u>https://doi.org/10.1016/j.jobe.2021.102977</u>.

reinforcement bars of grade BS 460B and BS B500B. The database also contains generated data 236 of energy dissipated in the first cycle, average cycles and total cycles of loading using numerical 237 integration of area enclosed by the stress-strain hysteresis loops. These experimental and 238 generated output parameters are function of: 1) Amplitudes of loading, A, and 2) strain ratio, R, 239 for steel grade of BS460B, BS B500B. This database was generated by Abdalla et. al [63], 240 Hawileh et. al [64] and has been used in ML studies to predict the fatigue life of steel reinforcing 241 bars [48]. Figure 5 and Table 6 show the distribution of all features comprising this database. 242 The outcome of the correlation matrix shows the highest positive correlation to be between 243 fatigue life (2Nf) and the total energy (W_{tT}), followed by R. A negative correlation exists between 244 2Nf and energy dissipated in the first cycle (W_1), energy dissipated in average cycles (W_2). Other 245 low-cycle fatigue of steel reinforcing bars databases were generated as a result of experimental 246 tests [65,66]. A sensitivity analysis was carried out to identify the correlation between all 247 features compiled in this database. The outcome of this analysis shows that all features seem to 248

be of high importance.



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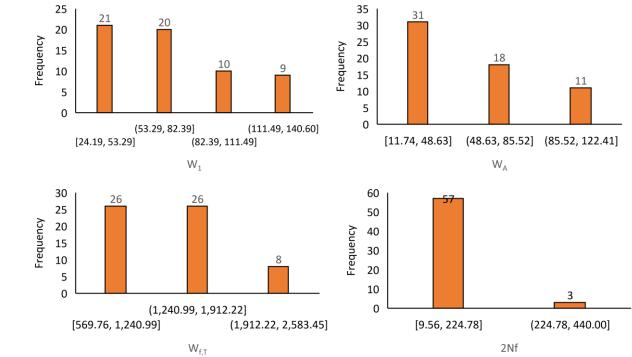




Fig. 5 Frequency of identified features of concrete mix designs in the compiled database

256	Table 6 Statistics on collected database
250	

Section	Features	A	R	W_{I}	W_A	$W_{f,T}$	$2N_f$
	Min	0.0	-1.0	24.2	11.7	569.8	9.6
	Max	0.1	0.0	140.6	122.4	2583.4	440.0
Compressive	Average	0.0	-0.6	72.9	55.2	1324.9	75.9
strength of HPC	Standard deviation	0.0	-0.6	30.4	27.8	482.7	75.7
	Median	0.0	0.4	0.6	0.6	0.6	2.6
	Skewness	0.4	0.3	24.2	11.7	569.8	9.6
Param	eter	A	R	W_1	WA	$W_{f,T}$	$2N_f$
Α		1.000					
R		-0.511	1.000				
W_{p1}	1	0.964	-0.344	1.000			
$\varDelta W_{p,i}$	avg	0.964	-0.561	0.958	1.000		
W_{fI}	r	-0.826	0.691	-0.763	-0.869	1.000	

Please cite this paper as:

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$2N_f$ -0.699 0.566 -0.638 -0.716 0.863

257

258 Database on shear strength of reinforced concrete (RC) beams and FRP-strengthened RC beams 259 (StructuresNet 5: Abdalla et al. database)

A comprehensive database that covers two types of reinforced concrete beams strengthened in shear with steel stirrups [55] and with externally-bonded carbon fiber reinforced polymer sheets [67]. The two databases collected 290 notable test results from several experimental programs to measure the shear strength of RC beams. These regression databases were used to predict the shear strength of RC beams using different ML techniques.

The first database documents geometric features in terms of: 1) beam width, b, 2) beam effective 265 depth, d, 3) span-to-depth ratio, a/d, 4) shear reinforcement ratio, ρ_v , 5) concrete compressive 266 strength, f_c , 6) flexural reinforcement ratio, ρ_w and 7) shear strength, $V_{n.}$ A graphical distribution 267 of all features in this database is plotted in Fig. 6 and Table 7 summarizes the main attributes of 268 the collected database in terms of material and geometric features. The second database 269 documents geometric features in terms of: 1) beam width, b_w , 2) beam effective depth, d_{eff} , 3) 270 beam span, L, 4) span-to-depth ratio, a/d, 5) concrete compressive strength, f_c , 6) steel yield 271 strength of stirrup, f_v , 7) shear reinforcement per length, A_v/S , 8) steel yield strength of 272 longitudinal reinforcement, f_y , 9) area of longitudinal reinforcement, A_{st} , 10) thickness of the 273 fiber, t_f , 11) width of the fiber, B_f , 12) height of the fiber, H_f , 13) width of the fiber over the 274 spacing ratio, W_f/S_f , 14) stress in the fiber, f_f , 15) modulus of elasticity of the fiber, E_f and 16) 275 shear strength of the beam, V_{f} . A graphical distribution of all features in this database is plotted 276 in Fig. 7 and Table 8 summarizes main attributes of the collected database in terms of material 277 and geometric features. The outcome of the correlation matrix shows the highest positive 278 correlation to be between the shear strength (V_n) and beam width (b) and between the shear 279 strength (V_n) and beam depth (d). A negative correlation exists between shear strength (V_n) and 280 span-to-depth ratio (a/d) and shear strength (V_n) and shear reinforcement ratio (ρ_v) . 281

Sensitivity analyses were carried out to identify the correlation between all features compiled in these databases. The outcome of these analyses shows that all features, d_{eff} , A_{v}/S , f_f , H_f , and E_f are

of the highest importance.

Please cite this paper as:

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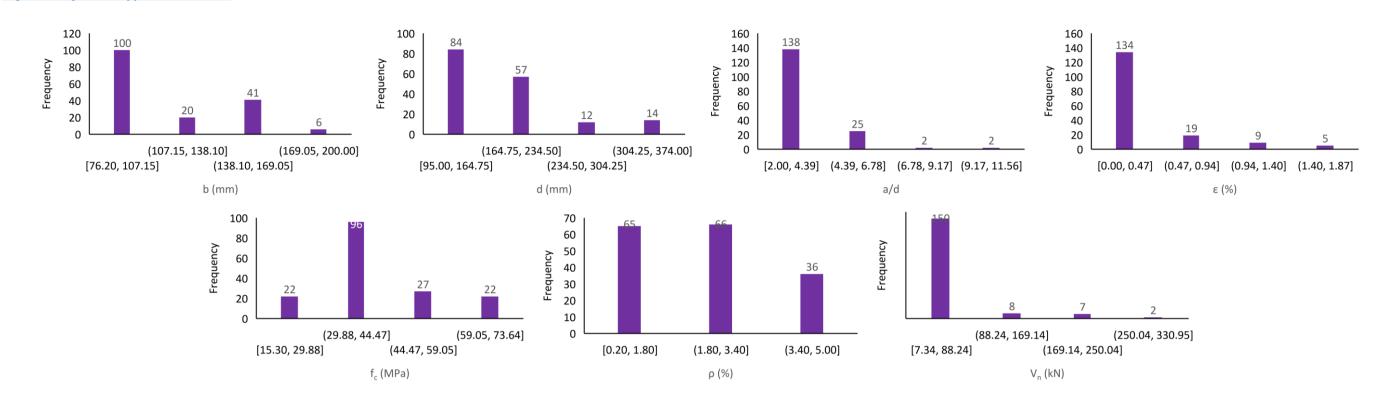
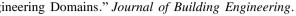


Fig. 6 Frequency of identified features of selected RC columns in the compiled database

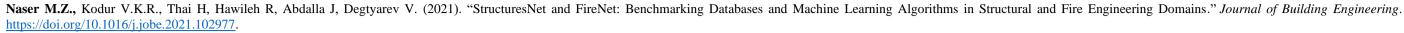
	Iuoie				Juse for rec	ocums		
Section	Features	b (mm)	d (mm)	a/d	$ ho_{\scriptscriptstyle V}$	f_c	ho	V_n
	Min	76.2	95.0	2.0	0.0	15.3	0.2	7.3
	Max	200.0	374.0	11.6	1.9	73.6	5.0	330.9
<u>C1</u>	Average	111.5	176.6	3.5	0.3	41.3	2.2	47.5
Shear strength of RC beams	Standard deviation	34.3	72.4	1.4	0.4	13.3	1.2	49.3
	Median	0.6	1.3	2.2	2.0	0.9	0.6	3.1
	Skewness	76.2	95.0	2.0	0.0	15.3	0.2	7.3
Param	Parameter b		d	a/d	$ ho_v$	f_c	$ ho_w$	V_n
b								
d		0.634	1.000					
a/d		-0.165	-0.223	1.000				
ρ_{v}		-0.501	-0.401	-0.070	1.000			
f_c		0.016	-0.080	0.078	0.040	1.000		
ρ_w		-0.309	-0.171	0.111	0.295	0.254	1.000	
V _n		0.527	0.506	-0.326	-0.191	0.181	0.353	1.000

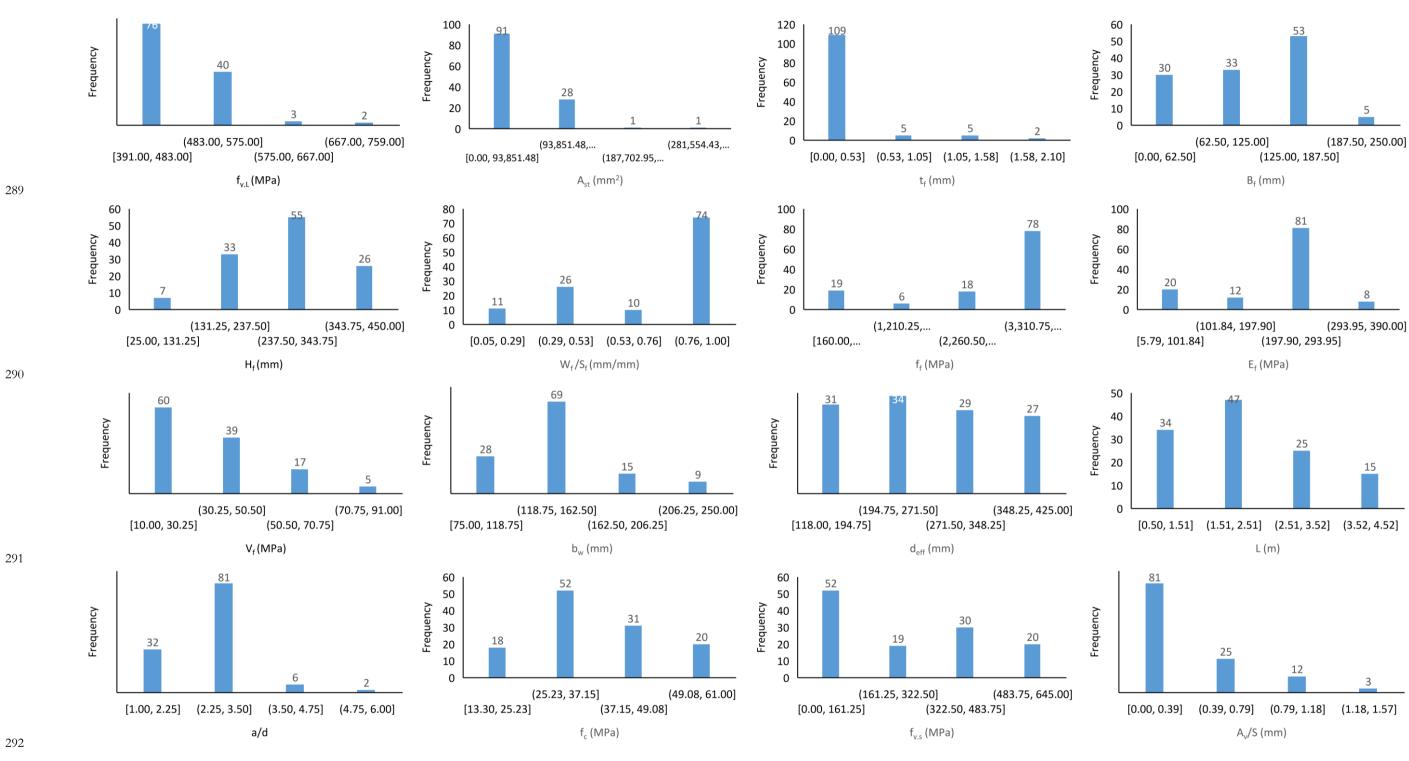
Table 7 Statistics on collected database for RC beams

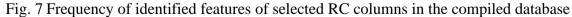
286 287



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	b_w (mm)	$d_{e\!f\!f}\(mm)$	L(m)	a/d	f_c (MPa)	$f_{y,s}$ (MPa)	A_{ν}/S (mm^2/mm)	$f_{y,L}$ (MPa)	$A_{st}(mm^2)$	t_f (mm)	B_f (mm)	$H_f(mm)$	W_{f}/S_{f}	f_f (MPa)	E_f (MPa)	$V_f(kN)$
Minimum	75.0	118.0	0.5	1.0	13.3	0.0	0.0	391.0	0.0	0.0	0.0	25.0	0.1	160.0	5.8	10.0
Maximum	250.0	425.0	4.5	6.0	61.0	645.0	1.6	759.0	375405.9	2.1	250.0	450.0	1.0	4361.0	390.0	91.0
Average	145.5	263.5	2.2	2.7	35.9	236.4	0.3	473.3	45958.4	0.3	104.7	262.9	0.8	2940.9	198.1	33.4
Standard deviation	41.9	84.7	1.0	0.8	10.3	222.8	0.4	65.6	66745.8	0.4	68.6	89.3	0.3	1223.2	92.8	17.9
Skewness	0.8	-0.2	0.8	1.3	0.4	0.1	1.4	1.5	1.7	2.8	-0.4	-0.6	-0.8	-1.4	-0.6	1.0
			_	_	_					_				_		
Parameter	b_w	$d_{\it eff}$	L	a/d	f_c	$f_{y,s}$	A_{ν}/S	$f_{y,L}$	A_{st}	t_f	B_f	H_{f}	W_{f}/S_{f}	f_{f}	E_{f}	V_{f}
b_w	1.000															
$d_{e\!f\!f}$	0.241	1.000														
L	-0.207	0.538	1.000													
a/d	-0.150	-0.131	0.108	1.000												
f_c	0.050	-0.059	-0.215	-0.076	1.000											
$f_{y,s}$	-0.174	0.141	0.108	-0.032	-0.025	1.000										
A_{ν}/S	-0.239	-0.031	0.069	-0.006	-0.034	0.657	1.000									
$f_{y,L}$	-0.056	0.185	-0.089	0.117	0.127	0.005	-0.107	1.000								
A_{st}	-0.196	-0.011	0.154	0.114	-0.076	0.723	0.882	-0.031	1.000							
t_f	-0.070	-0.301	-0.135	-0.047	0.151	0.041	0.075	-0.072	0.051	1.000						
B_f	0.107	0.148	0.106	0.178	0.119	0.100	0.101	0.122	0.134	-0.186	1.000					
H_{f}	0.258	0.857	0.357	-0.185	0.161	0.138	-0.149	0.199	-0.107	-0.376	0.108	1.000				
W_{f}/S_{f}	-0.244	-0.189	0.343	0.218	-0.267	0.245	0.194	-0.112	0.299	-0.087	0.086	-0.290	1.000			
f_{f}	0.179	0.351	0.281	-0.002	-0.187	-0.106	-0.216	0.032	-0.137	-0.521	0.168	0.490	-0.112	1.000		
E_{f}	0.239	0.359	0.206	0.017	-0.161	-0.095	-0.257	0.174	-0.144	-0.471	0.221	0.434	-0.132	0.819	1.000	
V_{f}	0.311	0.224	-0.023	-0.039	0.146	-0.155	-0.061	0.178	-0.152	-0.037	0.297	0.340	-0.260	0.189	0.128	1.000

295	Table 8 Statistics on collected database for FRP-strengthened RC beams	

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301 *Database on fire resistance of reinforced concrete columns (FireNet: Naser & Kodur database)*

The sixth database falls under a classification database and compiles real observations taken 302 from over 140 fire resistance tests (including spalling phenomenon) and 169 FE simulations on 303 reinforced concrete columns [68-81], and was compiled in [53]. This database contains 304 information on binary incidents of fire-induced spalling (i.e., column spalled/does not spall), 305 306 multi-class classification on fire rating of columns (e.g., in an hourly basis), and a regressionbased data (i.e., fire resistance duration). The identified features in the database include: 1) 307 column width, W, 2) steel reinforcement ratio, r, 3) column length, L, 4) concrete compressive 308 strength, f_c , 5) steel yield strength, f_v , 6) restraint conditions, K (fixed-fixed, fixed-pinned, and 309 pinned-pinned), 7) concrete cover to reinforcement, C, 8) eccentricity in applied loading in two 310 axes $(e_x \text{ and } e_y)$, 9) the magnitude of applied loading, P, and 10) fire failure time, FR. 311

Figure 8 and Table 9 present additional details into the range of each of the selected features. 312 Similar to the other databases, this database also covers a practical range of columns often used 313 in the construction industry. For example, all columns are of a square cross-section with a 314 minimum and maximum width between 203 mm and 601 mm. The steel reinforcement ratio 315 ranges between 0.9-4.4% and a length of 2.1-5.7 m. The range of yield strength of steel 316 reinforcement and compressive strength of concrete filling is from 354.0 MPa to 591.0 MPa, and 317 from 24.0 MPa to 138.0 MPa, respectively. The used concrete cover spans 25.0-64.0 mm and 318 eccentric between 0 and 150 mm. finally, the applied loading ranges between 0.0-5373.0 kN. 319

A sensitivity analysis was carried out to identify the correlation between all features compiled in this database. The outcome of this analysis shows a primarily weak correlation between the features and fire resistance except for the case of boundary conditions which displayed a medium negative correlation, a positive correlation attained by the concrete cover. In addition, a few interesting observations can also be made from this correlation analysis. For example, a high positive correlation appears to be between compressive strength and applied loading, and a medium correlation arises between column width and loading level. Please cite this paper as:

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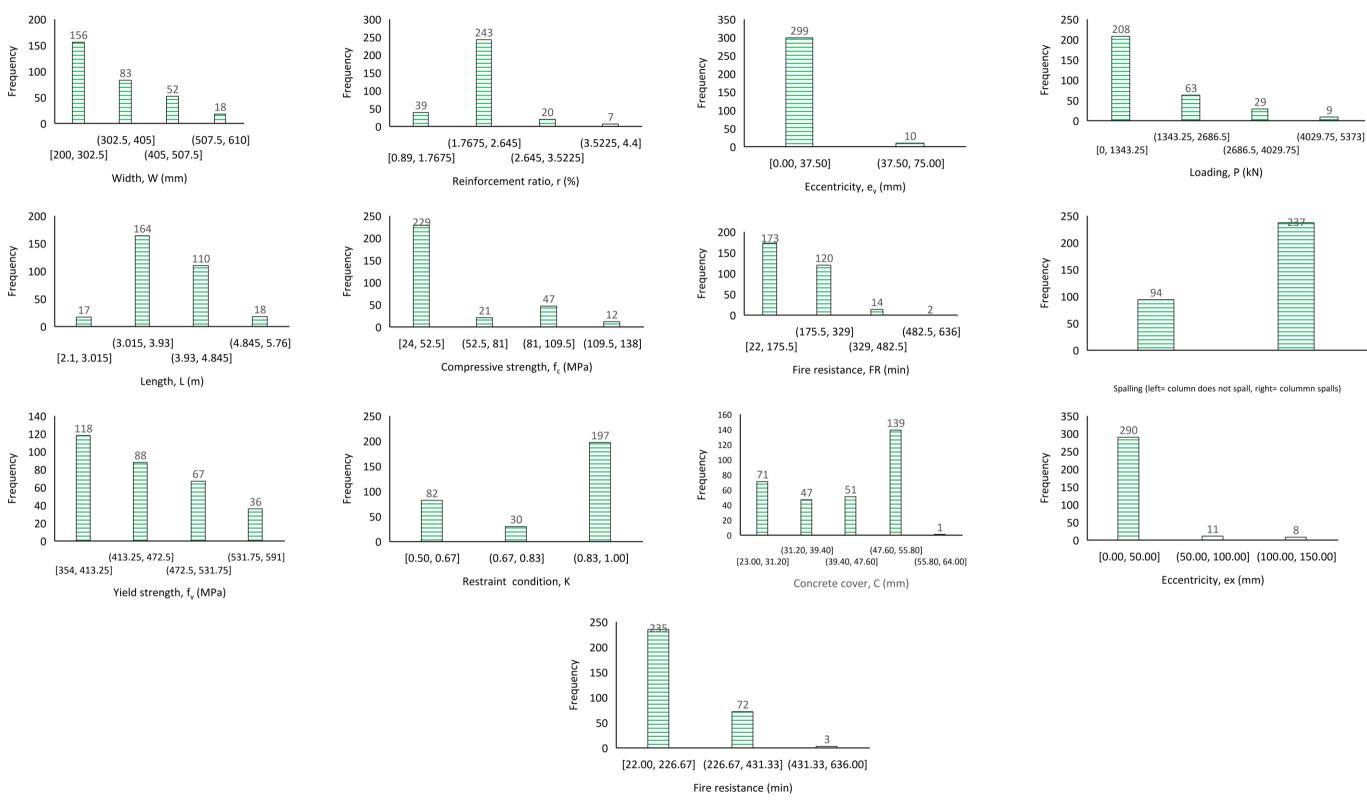


Fig. 8 Frequency of identified features of selected RC columns in the compiled database

327 328

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		W (mm)	r (%)	L(m)	fc (MPa)	$f_y(MPa)$	K	C (mm)	$e_x(mm)$	<i>e</i> y (<i>mm</i>)	P(kN)	FR (min)
Se	Minimum	200.0	0.9	2.1	24.0	354.0	-	23.0	0.0	0.0	0.0	22.0
anc	Maximum	610.0	4.4	5.8	138.0	591.0	-	64.0	150.0	75.0	5373.0	636.0
sist lysi	Average	324.3	2.1	4.0	49.3	449.4	-	40.2	15.8	2.0	1204.8	161.0
Fire resistance analysis	Standard deviation	99.2	0.6	0.7	28.1	60.1	-	8.7	29.7	10.1	1031.6	97.6
ц	Skewness	1.9	0.6	0.3	1.4	0.7	-	-0.6	2.9	5.3	1.7	0.9
	Minimum	152.0	0.7	-	16.0	-	-	25.0	-	-	0.0	-
B. 00	Maximum	514.0	4.9	-	126.5	-	-	64.0	-	-	5373.0	-
llin	Average	325.3	2.5	-	54.3	-	-	37.6	-	-	1556.9	-
Spalling Analysis	Standard deviation	69.4	0.8	-	27.9	-	-	4.4	-	-	1109.1	-
	Skewness	0.7	1.0	-	1.1	-	-	0.6	-	-	1.4	-
Par	rameter	W	r	L	f_c	f_y	K	С	e_x	e_y	Р	FR
	W	1.000										
	r	-0.120	1.000									
	L	-0.172	0.256	1.000								
	f_c	0.244	0.055	- 0.110	1.000							
	f_y	-0.250	-0.346	- 0.078	-0.478	1.000						
	Κ	0.022	-0.283	0.326	-0.079	0.169	1.000					
	С	0.319	0.312	- 0.224	0.279	-0.641	0.362	1.000				
	e_x	-0.088	0.046	0.356	-0.230	0.154	0.278	-0.257	1.000			
	e_y	0.156	-0.047	- 0.001	-0.136	-0.144	0.145	0.160	0.181	1.000		
	Р	0.670	0.121	- 0.206	0.559	-0.384	0.214	0.283	-0.213	0.035	1.000	
	FR	0.381	0.081	- 0.440	0.221	-0.277	- 0.604	0.558	-0.370	-0.043	0.365	1.000

330 Table 9 Statistics on collected database

331 Selected Machine Learning Algorithms

As mentioned earlier, the primary goal of this work is to benchmark commonly used ML 332 algorithms (in their default settings) against structural and fire engineering problems. In this 333 pursuit, a review of recent works [82-84] identified the following six algorithms as the most 334 commonly used algorithms in structural and fire engineering domains: Decision Trees (DT), 335 Random Forest (RF), Extreme Gradient Boosted Trees (ExGBT), Light Gradient Boosted Trees 336 (LGBT), TensorFlow Deep Learning (TFDL), and Keras Deep Residual Neural Network (KDP), 337 and these are briefly discussed herein. Most of these algorithms can be used in regression, and 338 classification problems which are expected to cover the majority of structural and fire 339 engineering problems. 340

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341 *Decision Trees (DT)*

The DT algorithm has the capability to generate a schematic representation of all possible 342 decisions and consequences, which can be visualized by dividing the database into branch-like 343 arrangements [85]. In general, a DT is generated and starts at a root node and then grows into 344 tree-like components (i.e., leaves etc.). The developed algorithm was obtained in its default 345 setting from Scikit platform [86]. This algorithm has a maximum depth of "none", minimum leaf 346 size and maximum size for split equals to 1 and "none", respectively [87,88]. This DT algorithm 347 utilized Gini impurity to facilitate the quality of a split and processing of datapoints. For 348 example, for a node t, the Gini index g(t) is defined as [89]: 349

$$g(t) = \sum_{j \neq i} p(j|t)p(i|t) \tag{1}$$

351 352

350

- 353 354
- where i and j are target field categories, and p is for probability.

355
$$p(j,t) = \frac{p(j,t)}{p(t)}; p(j,t) = \frac{\pi(j)N_j(t)}{N_j}; \text{ and } p(t) = \sum_j p(j,t)$$
 (2)

356

357 *Random Forest (RF)*

This algorithm integrates multiple DTs via ensemble learning to form a more powerful prediction model; hence, a forest of trees [90]. In RF, all individual DTs reach a predictive outcome. Then, this outcome is processed depending on the type of problem (i.e., regression vs. classification). For a regression problem, the average result of all trees is calculated to arrive at a final outcome. On the other hand, in a classification problem, the majority voting method is used to consolidate the final outcome. A typical formulation of RF is presented herein:

364 365

366

$$Y = \frac{1}{J} \sum_{j=1}^{J} C_{j,full} + \sum_{k=1}^{K} \left(\frac{1}{J} \sum_{j=1}^{J} contribution_j(x,k) \right)$$
(3)

where, *J* is the number of trees in the forest, *k* represents a feature in the observation, *K* is the total number of features, C_{full} is the average of the entire dataset (initial node). The used algorithm can be found herein [91] and has the following default settings; number of trees = 500, Gini impurity to facilitate quality of a split, a maximum depth of "none", minimum leaf size, and maximum size for split equals to 5 and "none", respectively.

372

373 Extreme Gradient Boosted Trees (ExGBT)

The ExGBT algorithm is an improved form of the Adaboost algorithm [92]. ExGBT re-samples the collected data points into a tree-like format, where each tree sees a bootstrap sample of the

database in each iteration. ExGBT fits each successive tree to previous residual errors obtained

from previous trees; thereby focusing each iteration on the observations that are most difficult to

predict, which becomes a good practice for the algorithm to yield high prediction accuracy [93].

The code of the used ExGBT can be found online at [94,95]. This algorithm incorporates default

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settings of a learning rate of 0.1, maximum tree depth of 3, subsample feature of 1.0, and 100 for
 the number of boosting stages.

- 382
- 383 Light Gradient Boosted Trees (LGBT)

Light gradient boosted trees is a light algorithm that requires little processing and is a generalization of their parent algorithm (Adaboost) [96]. This algorithm is very much similar to the RF algorithm with the main exception is that it does not fit the trees in parallel but rather it fits the trees in a successive manner and fits the residual errors from all the previous trees combined. This is advantageous, as the model focuses each iteration on the examples that are most difficult to predict. The used algorithm can be found at [97] with the following default settings: learning rate = 0.02, maximum depth = "none", number of boosting stages = 500 etc.

391 *TensorFlow Deep Learning (TFDL)*

This is a neural network-based model that uses Deep Learning as the primary method of analysis. A TFDL algorithm mimics the topology of the brain and comprises of a minimum of three layers. The first layer receives the database and forward it to the second set of layer(s). These layers use a nonlinear activation function which enables the algorithm of generating an approximation form that permits gradient-based optimization (see Eqs. 4 and 5). The used algorithm in its default settings (neurons in each layer = 55, number of training examples = 128, optimizer = Adam, learning rate = 0.001, early stopping window = 10 etc.) can be found at [98].

399
$$net_j = \sum_{i=1}^n In_i w_{ij} + b_j$$
 (4)

400

401 $Y = f(net_i)$

402

where, In_i and b_j are the *i*th input signal and the bias value of *j*th neuron, respectively, w_{ij} is the connecting weight between *i*th input signal and *j*th neuron, and *f* is an activation function such as Relu.

(5)

406

407 Keras Deep Residual Neural Network (KDP)

Keras is a high-level library for developing neural networks [99]. In a residual network, a direct connection exists linking data points to the outputs. Such a connection smoothens out the loss function and enables better optimization of the network. In the used KDP, default settings of a learning rate of 0.03 was used, along with a *Prelu* activation function, two layers containing 512 neurons. KDP can be readily found at [100].

413 Selected Performance Metrics

The adequacy of ML models in predicting engineering phenomena is often established through a comparison against performance metrics. Such metrics are defined as logical and/or mathematical constructs intended to measure the closeness of test measurements to that predicted by a ML model [101–103]. There exists a large body of literature covering a variety of metrics

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[104,105]. In this work, a focus is to provide the reader with a set of metrics that can be suitable
 for the majority of engineering applications. These metrics cover two domains, regression, and
 classification, as listed below.

In this study, four regression metrics and four classification metrics are presented (see Table 10). 421 These metrics are commonly used in structural and fire engineering literature [32,106–108]. On 422 the regression front, the metrics include; Mean Absolute Error (MAE), Mean Absolute 423 Percentage Error (MAPE), Root Mean Squared Error (RMSE), and Coefficient of Determination 424 (\mathbb{R}^2) . Both MAE and MAPE measure the difference between continuous variables in terms of the 425 same scale or as a percentage, respectively. MAPE tends to suffer when applied to predictions 426 with zero values. On the other hand, the RMSE describes the errors in a scale-independent 427 fashion, where lower values indicating favorable prediction capability. One should note that 428 RMSE is sensitive to outliers and to the fraction of the data used. R^2 is also used herein, and this 429 metric is the square of the coefficient of correlation (r); which measures the degree of association 430 between observed and predicted values with r closer to +1 indicates a positive and perfect linear 431 relationship. Higher and positive values of R^2 indicate strong and positive prediction capability. 432

433

On the classification front, four metrics are also presented, including; Accuracy (ACC), 434 Balanced accuracy (BACC), Area under the ROC curve (AUC), and Log Loss Error (LLE). 435 Unlike their regression counterparts, these metrics are used to evaluate the prediction capability 436 of a ML algorithm in terms of categorial outputs of binary (i.e., spalling occurs/spalling does not 437 occur), or multi-output classes (e.g., 60 minutes fire rating/120 min fire rating/180 minutes fire 438 ratings etc.). For instance, ACC evaluates the ratio of the number of correct predictions to the 439 total number of samples used in the analysis, and as such, assumes equal penalty for errors. 440 BACC is useful for databases with imbalanced data and multi-classes; where on class has 441 relatively larger occurrences than other classes. This metric is a normalized version of ACC and 442 calculates accuracy on a per-class basis, then averaging the per-class accuracies. The AUC 443 measures the area under the Receiver Operating Characteristic (ROC) curve; with a higher area 444 445 (close to 1.0) reflecting an accurate prediction capability. The Log Loss error measures the performance of a classification model whose output is a probability value between 0 and 1; 446 thereby, a prefect model would have a log loss of 0.0. 447

448

The above discussion shows that while all selected metrics have been used in engineering and 449 computer science benchmarking, they still tend to have some limitations, and hence it is 450 advisable to use a collection of metrics when evaluating ML algorithms in problems in our 451 domains. Comparing model performance across multi-metrics is seen of merit (as opposed a sole 452 metric) since this practice brings in a whole view to the performance of ML models. The ML user 453 is also advised as to apply due diligence in selecting proper metrics for the problem on hand. 454 For example, the use of regression-based metrics may not yield a proper exploration of 455 classification-based problems and vice versa. The above discussion covers key ideas behind 456 some of the most commonly used metrics and a more in-depth discussion on the provided 457

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458 *metrics, along with others such as Mean Squared Error (MSE), Reference index (RI), Confusion* 459 *Matrix (CM), and Cohen's kappa (CK) etc., can be found elsewhere* [104,105]. Please cite this paper as:

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Table 10 List of common performance metrics. 460

Table 10	List of common performance me	trics.				
Problem	<i>Name</i> Mean Absolute Error (MAE)	Metric Measures the difference between two continuous variables, as $MAE = \frac{\sum_{i=1}^{n} E_i }{n}$	 Uses a similar scale to input data [109]. Can be used to compare data points of diagonality. 			
Classification	Mean Absolute Percentage Error (MAPE)	Measures the extent of error in percentage terms, as $MAPE = \frac{100}{n} \sum_{i=1}^{n} E_i / A_i $	 Cannot be used if there are actual zero v Non-symmetrical (adversely affected if the corresponding actual value) [110]. 			
	Root Mean Squared Error (RMSE)	Measures the square root of the average of squared errors $RMSE = \sqrt{\frac{\sum_{i=1}^{n} E_i^2}{n}}$	 Scale dependent. A lower value for RMSE is favorable. Sensitive to outliers. Highly dependent on fraction of data user 			
	Coefficient of Determination (R ²)	Measures the goodness of fit of a mode $R^{2} = 1 - \sum_{i=1}^{n} (P_{i} - A_{i})^{2} / \sum_{i=1}^{n} (A_{i} - A_{mean})^{2}$	 R² values close to 1.0 indicate strong correlation. 			
	Accuracy (ACC)	Evaluates the ratio of number of correct predictions to the total number of samples. $ACC = \frac{TP + TN}{P + N} = \frac{TP + TN}{TP + TN + FP + FN}$	 Presents performance at a single class thr Assumes equal cost for errors [39]. 			
	Balanced accuracy (BACC)	$BACC = \frac{1}{M} \sum_{m=1}^{M} \frac{r_m}{n_m}$ where, M = number of classes, n_m data size belongs to class m , r_m =number of data accurately predicted belonging to class m .	 Balanced accuracy is a metric that one ca or multi-classifier is. Useful for imbalanced and multi-classifier 			
	Area under the ROC curve (AUC)	Measures the two-dimensional area underneath the entire ROC curve. $AUC = \sum_{i=1}^{N-1} \frac{1}{2} (FP_{i+1} - FP_i) (TP_{i+1} - TP_i)$	 Not dependent on a single class threshold Associated with increased training times. 			
	Log Loss Error (LLE)	Measures the where the prediction input is a probability value. $LLE = -\sum_{c=1}^{M} A_i logP,$ where, M: number of classes, c: class label, y: binary indicator (0 or 1) if c is the	 Penalizes for being too confident in wrom Has probability between zero and 1. A log loss of zero indicates a perfect mod 			
A· actual	measurements <i>P</i> predictions <i>n</i>	correct classification for a given observation. : number of data points, $E = A - P$. <i>P</i> (denotes number of real positives). <i>N</i> (denotes number of real positives).	umber of real negatives) TP (denotes true			

A: actual measurements, P: predictions, n: number of data points, E = A-P, P (denotes number of real positives), N (denotes number of real negatives), TP (denotes true positives), TN (denotes true negatives), FP 461

(denotes false positives), and FN (denotes false negatives). 462

marks
lifferent scales.
values. a predicted value is larger or smaller than
ed (low reliability) [111].
rrelation.
nreshold only.
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ication databases.
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463 Benchmarking of selected algorithms

This section details the benchmarking of all selected algorithms at the six compiled databases. 464 As mentioned above, all algorithms were used given their default settings to allow a raw 465 evaluation of their performance against structural and fire engineering data/problems. Table 11 466 lists the outcome of the carried-out benchmark analysis in terms of performance metrics under 467 training, validation and testing regimes. All analyses adopted a five-fold cross-validation 468 procedure. The best performing algorithms are shown in bold in Table 11. For simplicity and to 469 negate the notion of chasing accuracy as it is beyond the objective of this work, all results were 470 rounded for two decimal places. 471

As one can see and as expected, not a single algorithm was found to be dominant in all of the 472 carried-out examinations, nor in all three testing regimes. This highlights the need for adopting 473 multiple algorithm search, and multiple performance metrics in a given ML analysis. One should 474 still note that of all algorithms, ExGBT and LGBT seem to outperform all other algorithms, with 475 ExGBT leading. For instance, ExGBT managed to score the best metrics in Database 1, 3, 4 and 476 5, while LGBT performed comfortably well in Database 2. On the contrary, the DT algorithm 477 performed the poorest of all algorithms in the majority of the tested databases, followed by RF 478 479 and TFDL.

A note to remember is that the outcome of this analysis only reflects upon the selected six algorithms and does not imply that other algorithms may not perform better than those used herein. The same also goes for the selected performance metrics. As mentioned earlier, the notion of this work is not to start an "accuracy chase", especially since, as the conducted analysis shows, accurateness is not only a complex metric to realize and achieve but is also subjective and requires a deep dive into multi-metrics and domains. Please cite this paper as:

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486 Table 11 Outcome of benchmark analysis

Table 11 Ou	atcome of ben	ichmark an	nalysis																
	Metric		DT			RF			ExGBT			LGBT			TFDL			KDP	
	Metric	Training	Validation	Testing	Training	Validation	Testing	Training	Validation	Testing	Training	Validation	Testing	Training	Validation	Testing	Training	Validation	Testing
Database 1 (Regression)	\mathbb{R}^2	0.96	0.96	0.87	0.82	0.94	0.83	0.99	0.99	0.92	0.91	0.96	0.99	0.57	0.56	0.39	0.86	0.93	0.88
	MAE	291.48	280.57	437.52	331.19	331.41	425.11	115.18	132.13	235.20	197.91	161.74	116.06	835.06	926.84	881.29	369.90	298.70	366.28
	MAPE	12.69	13.27	13.31	12.99	15.77	12.77	6.29	6.95	6.73	7.00	6.72	5.65	29.58	36.52	26.26	11.99	11.37	10.50
D£ (R¢	RMSE	707.966	691.71	1665.5	1005.55	881.29	1929.42	260.81	<i>309.48</i>	1293.29	1091.33	646.86	254.28	2433.55	2493.91	3654.07	1374.04	928.01	1608.60
2 (1	\mathbb{R}^2	0.95	0.95	0.94	0.92	0.93	0.94	0.99	0.99	0.99	0.99	0.91	0.99	0.33	0.73	0.81	0.99	0.93	0.99
ase Z	MAE	3966.50	3964.98	3699.48	5399.59	4838.66	4321.92	1844.14	1695.84	1625.74	1528.76	1519.08	1552.55	8326.67	9069.17	7703.48	2242.65	2200.97	1977.25
taba gree	MAPE	12.09	11.78	11.50	20.27	19.16	18.67	5.25	5.0	4.87	4.47	4.60	4.48	26.08	28.38	26.75	6.55	6.63	6.57
Database 2 (Regression)	RMSE	8688.51	8803.70	8861.78	11547.00	9965.00	922.20	4165.88	3727.08	4012.00	3492.27	3489.72	3756.76	16313.00	17212.00	16263.00	4799.37	4770.15	4235.65
	\mathbb{R}^2	0.88	0.83	0.82	0.89	0.88	0.89	0.93	0.92	0.94	0.69	0.64	0.67	0.48	0.46	0.39	0.88	0.89	0.90
sion	MAE	4.44	5.05	4.70	4.18	4.18	4.03	3.10	3.11	2.66	7.56	8.056	7.65	10.57	10.73	10.82	3.85	3.77	3.43
taba gres	MAPE	15.83	17.85	15.01	15.27	14.94	13.64	10.90	10.59	8.91	35.34	34.42	29.36	29.93	31.97	30.86	12.69	12.14	11.46
Database 3 (Regression)	RMSE	5.97	6.81	6.67	5.46	5.70	5.25	4.23	4.59	3.7	9.42	10.08	9.08	12.37	12.39	12.43	5.88	5.64	4.98
	\mathbb{R}^2	-1.00	0.42	0.69	0.56	0.72	0.88	0.59	0.78	0.90	-1.00	0.22	0.62	-3.02	-0.54	0.58	-0.96	0.27	0.71
sion	MAE	24.50	20.16	24.28	15.17	18.27	17.84	11.05	18.15	14.02	28.01	26.49	31.21	54.31	42.92	36.78	25.60	27.40	22.88
Database4 (Regression)	MAPE	41.93	24.26	4099	48.65	27.32	35.19	26.29	22.05	28.04	237.59	97.59	133.92	74.33	37.63	49.84	54.81	37.76	38.44
	RMSE	44.29	37.29	37.33	20.47	34.78	22.87	<i>19.63</i>	30.10	21.32	44.67	48.55	41.56	68.29	63.18	43.76	43.15	46.87	36.25
	R ²	0.40	0.72	0.47	0.75	0.80	0.25	0.06	0.05	0.01	0.09	0.06	0.80	0.91	0.90	0.54	0.02	0.05	0.04
ion)	MAE	0.49 17.39	13.90	12.72	0.75 13.58	0.80	0.35 13.88	0.96 5.75	0.95 5.70	0.91 5.71	0.98 3.13	0.96 5.11	0.80 7.18	0.81 12.18	0.80	0.54 12.76	0.98 3.46	0.95	0.94 4.44
Database5 (Regression)	MAPE	66.19	37.72	37.32	47.79	34.72	51.72	1864	14.36	19.16	8.06	11.67	27.36	36.32	40.00	43.79	7.69	10.61	15.90
	RMSE	28.70	23.32	24.09	20.42	20.39	26.60	7.62	9.14	9.93	4.83	9.23	14.78	17.53	19.46	22.39	5.28	9,97	7.97
	KNDL	20.70	23.32	24.09	20.42	20.39	20.00	7.02	7.14	9.93	4.03	9.23	14.70	17.55	19.40	22.39	J.20	9,97	1.91
	R ²	0.1	0.01	0.12	0.22	0.26	0.06	0.50	0.29	0.20	0.27	0.22	0.21	2.00	2.17	2.00	0.24	0.29	0.21
e5 on)	MAE	-0.1 12.32	0.01 13.19	0.12 14.64	0.32	0.26	0.06 13.97	0.50 8.48	0.28	0.29 12.13	0.37 9.29	0.22	0.21 12.69	-2.99 25.81	-2.17 25.49	-2.00 28.26	0.34 9.61	0.28 12.11	0.31 12.10
ıbas	MAPE	50.48	50.16	71.03	40.99	45.18	67.62	34.50	44.11	54.49	39.74	44.05	63 .27	84.95	75.26	85.53	38.12	45.25	62.30
Database5 (Regression)																			
I (I)	RMSE	14.96	17.02	19.71	11.73	14.65	17.96	10.11	1438	15.63	11.32	14.96	16.47	28.53	30.14	32.57	11.67	14.44	15.39
e (uc	AUC	0.78	00.74	0.82	0.82	0.80	0.84	0.83	0.80	0.87	0.81	0.78	0.80	0.81	0.79	0.83	0.81	0.76	0.80
ase -cla: catie	Accuracy Balanced	0.30	0.42	0.67	0.52	0.46	0.57	0.47	0.42	0.71	0.47	0.43	0.42	0.43	0.5	0.42	0.43	0.46	0.46
Database 6 (Multi-class Classification)	Accuracy	0.36	0.38	0.62	0.49	0.44	0.51	0.48	0.43	0.67	0.45	0.42	0.42	0.42	0.48	0.37	0.42	0.44	0.42
	Log Loss	4.01	4.04	4.38	1.16	1.20	1.09	1.15	1.20	0.97	1.15	1.24	1.17	1.19	1.31	1.18	1.21	1.48	1.43
ase ary ïca	AUC	0.69	0.65	0.78	0.76	0.82	0.86	0.77	0.85	0.87	0.79	0.77	0.85	0.76	0.73	0.74	0.76	0.80	0.76
Database 6 (Binary Classifica tion)	Accuracy	0.67	0.70	0.76	0.77	0.76	0.84	0.78	0.79	0.84	0.77	0.78	0.81	0.74	0.76	0.79	0.78	0.78	0.79
Da 6 (Cl	Balanced	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-

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	Accuracy																		
	Log Loss	0.61	0.88	0.52	0.60	0.51	0.43	0.64	0.48	0.48	0.52	0.52	0.48	0.54	0.56	0.58	0.65	0.55	0.71
ise 6 sion)	R ²	0.44	0.58	0.32	0.54	0.69	0.50	0.73	0.77	0.45	0.57	0.64	0.49	0.51	0.51	0.41	0.48	0.65	0.29
DaS (MAE	45.15	43.20	45.93	41.00	36.54	34.42	30.76	30.51	33.26	39.21	40.36	38.72	41.18	46.47	50.01	41.79	37.70	42.88
Databa: (Regress	MAPE	27.15	32.15	32.92	27.13	27.21	29.19	20.16	22.42	21.53	28.54	30.55	34.20	30.04	37.93	44.42	28.01	26.57	28.92
	RMSE	67.63	60.25	91.1	61.47	50.80	77.43	47.29	43.86	81.38	59.49	56.10	78.27	63.33	64.66	84.37	64.81	54.78	92.30
Leaderboard	Recurrence	0	1	0	1	2	3	24	22	17	10	9	9	0	2	0	1	2	8
	%	0	2	0	2	5.6	8.3	<u>66.7</u>	<i>61.2</i>	47	30.5	19.4	25	0	5.6	0	2	5.6	22

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487 **Future directions**

This benchmarking study alludes to the notion that intentional, or unintentional cherry-picking in 488 a given ML analysis is likely; given that not all users are familiar with all different ML modeling 489 techniques, nor there is a requirement to attempt to try to examine all possible ML techniques. In 490 the majority of scenarios, and rightly so, a user may in fact favor algorithms that s/he familiar 491 with over others. As such, properly benchmarking ML model is needed now noting how the use 492 of ML into our domains is expected to continue to rise and hence works targeting benchmarking 493 494 will set the foundation towards a reliable and safe integration of this new technology. Early attempts in this area will help overcome existing issues related to standardization and validation 495 of FE models, among other methods [7,112]. In addition, future attempts will continue to 496 overcome some of the current limitations of ML especially those with regard to limited number 497 498 of data points, selection of tuning parameters, different coding languages, need for improved inference performance etc. [82,83]. 499

One could argue that modeling (in general) may not be suited for technicians, like testing 500 standards for materials, and hence a FE model should only be implemented by trained 501 engineers. However, a trained engineer is also required to follow/adhere to a procedure. To 502 ensure compatibility, such a procedure is to be unified, generally accepted, or standardized for 503 repeatability and transparency. We, then, argue that the modeling, whether is to be deployed by 504 technicians or engineers, also needs to follow a commonly accepted procedure. In a way, a move 505 towards a unified procedure will facilitate both inclusivity and diversity into our domains. Such 506 a procedure can start by benchmarking commonly used ML models as it is customary in the 507 computer science domain [113–115]. The message of this work also aligns with that proposed by 508 other researchers that focused on unifying FE modeling procedures [24–27]. 509

This paper focuses on benchmarking commonly available ML algorithms against structural and 510 fire engineering phenomena by analyzing six notable databases that have been properly 511 documented and examined in the open literature. As such, the primary goal of this paper is not to 512 chase high accuracy scores but rather establishes a benchmark for the following ML models DT, 513 RF, ExGBT, LGBT, TFDL, and KDP against structural and fire engineering problems. Similar to 514 other works [116–119], we hope that *StructuresNet* and *FireNet* can accelerate the use of ML 515 into the structural engineering and fire engineering domains. In the future to come, new works 516 are encouraged to cross-check their ML models' predictive power against findings from this 517 benchmarking study. We expect finetuned upcoming ML models to achieve improved 518 performance than what we displayed herein. Interested works are also invited to continue 519 progress in this area as a mean to capitalize upon the attractiveness of ML. 520

There are three sub-domains to benchmarking: 1) number and types of databases, 2) used performance metrics, and 3) repeatability of predictability [120,121]. This paper covers the first two sub-domain, and as such, work is needed to benchmark the latter by examining derivates of feature selection techniques, model tuning parameters (in terms of the learning rate, loss functions, activation functions, hyperparameter tuning etc.), use of optimizers, hybrid and ensemble modeling approaches. For example, Degtyarev [32] showed how finetuning some of

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the noted parameters above can result in large improvements (whether in terms of shorter processing time, or attaining higher accuracy metrics). In addition, one must not forget benchmarking hardware or cloud services associated with ML modeling as well. Such benchmarking may lead to developing eco-friendly or green ML models that do not require intense energy resources to solve structural or fire engineering problems.

532 Conclusions

This paper presents a framework for developing, benchmarking, and validating commonly 533 adopted supervised learning ML algorithms against databases compiled for structural and fire 534 engineering problems. These presented datasets cover six domains, 1) elemental response of 535 CFST circular CFST columns at ambient conditions, 2) shear response of CFS channels with 536 slotted webs, 3) compressive strength of concrete, 4) fatigue life data, 5) shear strength of RC 537 and FRP-strengthened beams and fire engineering; and 6) fire behavior of RC concrete columns 538 in terms of spalling occurrence and fire resistance. In total, six algorithms were benchmarked 539 including; Decision Trees (DT), Random Forest (RF), Extreme Gradient Boosted Trees 540 (ExGBT), Light Gradient Boosted Trees (LGBT), TensorFlow Deep Learning (TFDL), and 541 Keras Deep Residual Neural Network (KDP). Holistically, the presented paper establishes the 542 first step towards a unified framework that can be used to accelerate the adoption of ML into 543 structural and fire engineering domains. 544

- 545 The following list of inferences can also be drawn from the findings of this study:
- All selected algorithms in their default settings seem to properly capture the structural and fire engineering phenomena examined herein (with satisfactory and varying levels of success). This implies that structural and fire engineers can adopt raw algorithms as is, as opposed to developing complex ML models or undergo painful programming exercises. This also implies that complications arising due to engineers' historically limited knowledge on ML coding (given the lack of ML presentations into structural and fire engineering curriculum) can be easily overcome.
- Of all algorithms showcased herein, both Extreme Gradient Boosted Trees (ExGBT), Light Gradient Boosted Trees (LGBT) seem to rank the highest on the carried-out tests.
- As expected, out of all examined algorithms, not a single algorithm was found to be dominant in all of the carried-out examinations. This highlights the need for adopting multiple algorithm search and multiple performance metrics in a given ML analysis.
- Benchmarking efforts are encouraged to continue to develop accepted databases and performance evaluations of ML algorithms since the integration of ML into our domains is on the horizon. Early efforts will not only ensure a smooth transition into automation within our historically slow-adapting fields but will also negate existing hurdles observed in attempting to unified FE simulation methods.

563 Data Availability

564 Some or all data, models, or code that support the findings of this study are available from the 565 corresponding author upon reasonable request. All of the presented databases are hosted online 566 on public repositories (*and complete links to these databases are shown herein*).

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- 567 Links to databases:
- 568 Database 1 [59]. Database 2 [122]. Database 3 [123]. Database 4 [63,64]. Database 5 [55,67].
- 569 Database 6 [54].

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